

Compression perpendicular to grain (bearing)

$$F_{c\perp u} = F_{c\perp} (K_F) = 565 \text{ psi } (2.083) = 1177 \text{ psi} = 1.177 \text{ ksi}$$

$$F_{c\perp u}^* = F_{c\perp u} (\Phi_c)(\lambda) = 1.177 \text{ ksi } (0.9)(0.8) = 0.847 \text{ ksi}$$

Compression parallel to grain

$$F_{cu} = F_c (K_F) = 1800 \text{ psi } (2.4) = 4320 \text{ psi} = 4.32 \text{ ksi}$$

$$F_{cu}^* = F_{cu} (\Phi_c)(\lambda) = 4.32 \text{ ksi } (0.9)(0.8) = 3.11 \text{ ksi}$$

$$P_u^* = F_{cu}^* (A) = 3.11 \text{ ksi } (39.38 \text{ in}^2) = 122 \text{ k}$$

*Note that  $C_F$  would also apply for column buckling of an axially loaded member.*

Modulus of Elasticity

$$E^* = E = 1,800,000 \text{ psi} = 1800 \text{ ksi}$$

$$E_{min-u} = E_{min} (K_F) = 660,000 \text{ psi } (1.765) = 1,165,000 \text{ psi} = 1165 \text{ ksi}$$

$$E_{min-u}^* = E_{min-u} (\Phi_s) = 1165 \text{ ksi } (0.85) = 990 \text{ ksi}$$

c. **2 x 10 purlins at 4 ft o.c.; 1.2D + 1.6L<sub>r</sub> load combination ( $\lambda = 0.8$ )**

$C_F$  is already incorporated in NDS Table 4B values; other factors equal to unity

$$A = 13.88 \text{ in}^2; S_x = 21.39 \text{ in}^3$$

Bending moment

$$F_{bu} = F_b (K_F) = 2050 \text{ psi } (2.54) = 5207 \text{ psi} = 5.21 \text{ ksi}$$

$$F_{bu}^* = F_{bu} (\Phi_b)(\lambda) = 5.21 \text{ ksi } (0.85)(0.8) = 3.54 \text{ ksi}$$

$$M_u^* = F_{bu}^* (S_x) = 3.54 \text{ ksi } (21.39 \text{ in}^3) = 75.8 \text{ k-in.}$$

Tension

$$F_{tu} = F_t (K_F) = 1100 \text{ psi } (2.7) = 2970 \text{ psi} = 2.97 \text{ ksi}$$

$$F_{tu}^* = F_{tu} (\Phi_t)(\lambda) = 2.97 \text{ ksi } (0.8)(0.8) = 1.90 \text{ ksi}$$

$$T_u^* = F_{tu}^* (A) = 1.90 \text{ ksi } (13.88 \text{ in}^2) = 26.4 \text{ k}$$

Flexural Shear

$$F_{vu} = F_v (K_F) = 175 \text{ psi } (2.88) = 504 \text{ psi} = 0.504 \text{ ksi}$$

$$F_{vu}^* = F_{vu} (\Phi_v)(\lambda) = 0.504 \text{ ksi } (0.75)(0.8) = 0.302 \text{ ksi}$$

$$V_u^* = F_{vu}^* (2/3)(A) = 0.302 \text{ ksi } (2/3)(13.88 \text{ in}^2) = 2.8 \text{ k}$$

Compression perpendicular to grain (bearing)

$$F_{c\perp u} = F_{c\perp} (K_F) = 565 \text{ psi } (2.083) = 1177 \text{ psi} = 1.177 \text{ ksi}$$

$$F_{c\perp u}^* = F_{c\perp u} (\Phi_c)(\lambda) = 1.177 \text{ ksi } (0.9)(0.8) = 0.847 \text{ ksi}$$

Compression parallel to grain

$$F_{cu} = F_c (K_F) = 1850 \text{ psi } (2.4) = 4440 \text{ psi} = 4.44 \text{ ksi}$$

$$F_{cu}^* = F_{cu} (\Phi_c)(\lambda) = 4.44 \text{ ksi } (0.9)(0.8) = 3.20 \text{ ksi}$$

$$P_u^* = F_{cu}^* (A) = 3.20 \text{ ksi } (13.88 \text{ in}^2) = 44.4 \text{ k}$$

*Note that  $C_F$  would also apply for column buckling of an axially loaded member.*

Modulus of Elasticity

$$E^* = E = 1,800,000 \text{ psi} = 1800 \text{ ksi}$$

$$E_{min-u} = E_{min} (K_F) = 660,000 \text{ psi } (1.765) = 1,165,000 \text{ psi} = 1165 \text{ ksi}$$

$$E_{min-u}^* = E_{min-u} (\Phi_s) = 1165 \text{ ksi } (0.85) = 990 \text{ ksi}$$

# Design Of Wood Structures 6th Edition Solutions Manual

**Katrin Zwirgmaier**



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