

CodeMaster



STRUCTURAL
WOOD DESIGN
ASD/LRFD

2018
IBC

ASCE
7-18

2018
NDS

STRUCTURAL DESIGN OF WOOD MEMBERS

This CodeMaster provides ten steps to guide the engineer in structurally designing typical wood framing members (columns, beams, joists, rafters, etc.) in accordance with the 2018 International Building Code (IBC), ASCE 7-18, Minimum Design Loads and Associated Criteria for Buildings and Other Structures (ASCE 7), and the 2018 National Design Specification® for Wood Construction (NDS®). The NDS includes a valuable commentary and an NDS Supplement which provides section properties and reference design values. Allowable Stress Design (ASD) is emphasized; however, Load and Resistance Factor Design (LRFD) is also included. This CodeMaster addresses the design of rectangular sawn lumber and structural glued-laminated timbers (GLT). Wood structural panels, wood I-joists, structural composite lumber (CL, PCL, LSL, OSB, etc.), cross-laminated timber (CLT) and decking are not addressed.

Wood has a wide array of different material properties in separate orthogonal directions that are influenced by many factors. Recognizing wood's unique properties and proper application of adjustment factors is at the heart of timber design. Calculated stresses are denoted by lower case letters (such as f_x for calculated bending stress) and reference design values by upper case letters (such as F_b). The NDS denotes the adjusted design values with a "prime" marking (F'_b , F'_c , F'_t , F'_e , F'_{ax} , E' , E'_{min} , etc.), and it is these adjusted values that are the basis for design.

STEP 1 DETERMINE LOADS AND LOAD COMBINATIONS

Wood structural members such as beams and columns are subject to many types of loads, including gravity loads (e.g., dead, live, snow, and rain) and lateral loads (e.g., wind, earthquake, and soil). Gravity loads are determined by computing the structure's self-weight in addition to live and snow loads determined in accordance with IBC Sections 1605, 1607 and 1608. Wind loads are determined in accordance with IBC Section 1609 and ASCE 7 Chapters 28 through 31. Earthquake loads are determined in accordance with IBC Section 1613 and ASCE 7 Chapters 11, 12, 13, 15, 17, 18. See the CodeMasters on wind and seismic design for more information. Once the loads have been determined, the load path through the structure is analyzed to determine its effects such as bending moments, shear forces, compression forces, and tension forces on individual framing members. The load effects are then combined in accordance with the load combinations of IBC Section 1605.

SECRETS OF THE CODEMASTER: The self-weight of wood framing may be assumed to be 35 lb/ft² for most structural lumber at typical equilibrium moisture content. NDS Supplement Table 1B provides self weights for common member sizes. However, published dead loads assumed for certain applications (e.g., residential) often already include self-weight of sawn lumber framing members.

SECRETS OF THE CODEMASTER: Designers have a choice of using ASD or LRFD load combinations. While ASD is more conventional, designs using LRFD loads are easily adaptable with simple conversion factors and with most adjustments remaining unchanged.

STEP 2 DETERMINE AXIAL, SHEAR, AND BENDING FORCES

Using unit loads from STEP 1 in conjunction with tracing the load path, member bending, shear, compression, and tension forces are determined. Computing member tributary area is critical to load path analysis and is also necessary in determining any applicable live load reductions. Tributary area live load reductions are determined in accordance with IBC Section 1607.11 (floor live) and Section 1607.13.2 (roof live). Where applied beam loads occur near the member's support, the design shear force may be reduced per NDS Section 3.4.3.1.

Figure 1. Roof of Pined Prairie Post

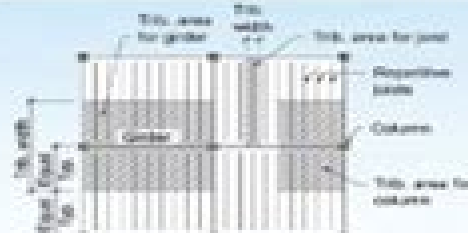
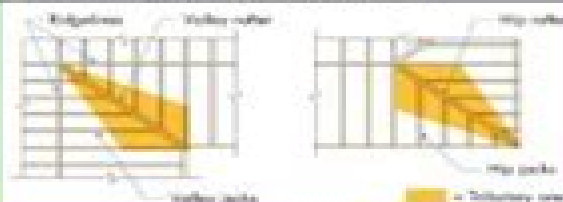


Figure 2. Tributary Areas of Valley and Hip Rafters



SECRETS OF THE CODEMASTER: For sloping bending members such as roof rafters, roof live loads and snow loads are defined as horizontal-projected plane loads, and it is often acceptable to simply use horizontal span lengths for the bending member analysis. While this simplification ignores the increased length of the rafter relative to a horizontal projection, it also ignores the additional bending and compression load capacity provided by the roof sheathing and the reduced loads using a sloped rafter length versus the horizontal projection.

Figure 3. Typical Rafter Span Simplification



STEP 3 SELECT MATERIALS TO BE USED

Selection of sawn lumber member size and wood species group is largely driven by regional availability. For a given location, only a few species groups will generally be available. Most common are the following softwood species groups:

Douglas fir-larch
Southern pine
Hem-fir
Spruce-pine-fir

Most typically specified are dressed sizes which are $\frac{1}{8}$ inch to $\frac{1}{4}$ inch smaller than specified nominal sizes. The range of dressed sizes and their section properties are in NDS Supplement Table 1B. Dimensional size terminology defines thickness (breadth) as the narrow dimension and width (depth) as the largest dimension.



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